Household Shelter Structural Requirement Checklist:

- (1) The essential technical requirements that have impacts on the structural safety, performance and protection levels of the household shelters are listed in the Table.
- (2) The list shall be used as a guide to check that the plan submitted fulfils essential technical requirements.
- (3) For other technical requirements not in the list, they shall be referred to Technical Requirements for Household Shelters 2017.

Disclaimer: This checklist serves as a guide to qualified person in relation to the preparation of (i) Building Plans which will incorporate the architectural aspects of Civil Defence ("CD") Shelter Plans and (ii) Structural Plans which will incorporate the structural aspects of the CD Shelter Plans and are for informational purposes only. This checklist does not purport to be exhaustive or applicable to all situations and does not constitute professional advice. The Building and Construction Authority disclaims any liability (including any liability arising from negligence) arising in respect of any matter and the consequences of any act done or not done by any person in reliance on anything in or omitted from this checklist.

1	Material	Clause or Table or Fig. in TRHS 2017
	Concrete	
	Landed development: Min. C25/30 Non-landed Landed development: Min.C32/40 Normal concrete density (2400kg/m³) and consists of 20mm nominal maximum size coarse aggregate, apply to all HS wall (include hollow core & joints, RC elements within setback distance)	Cl. 3.2.1
	Steel reinforcement Welded steel fabric mesh for steel bar Yield stress Min. 500 N/mm²	Cl. 3.2.2
2	Analysis (a) Shielded NS walls and/ or NS columns - No additional design check if supporting NS wall/ NS columns are shielded by RC slab	Cl. 3.3.4 (a)
	(b) Unshielded NS walls and/ or NS columns - Min NS wall 300 mm thk; Min NS Column 500 mm thk Additional design check against the most severe effects of the removal of a portion of the NS wall equivalent to 1.5m opening (@ most critical position) Additional design check against the most severe effects of the removal of one NS column at a time Design load and design strength of material to be adopted shall be 1.2 for concrete & 1.0 for steel	Cl. 3.3.4 (b)-(d) Fig. 3.3.4(a)-(c) Table 3.3.4 (d)
	(c) Shielded Transfer structure supporting HS tower No additional design check on transfer structure if transfer structure is adequately shielded	Cl 3.3.5.2 (a)-(b) Fig. 3.3.5.2 (a)-(b)
	(d) Unshielded Transfer structure supporting HS tower	
	Unshielded transfer slab/ Beams Additional design check against the most severe effects of removal of a portion of the transfer slab/beam equivalent to 1.5m opening (@ most critical position)	Fig. 3.3.5.3(a)(i) & (ii)
	Unshielded exterior columns/ walls Min column 500mm thk & wall 300 thk wall Additional design check against the most severe effects of the removal of a portion of the exterior wall equivalent to 1.5m opening (@ most critical position) Additional design check against the most severe effects of the removal of an exterior column at a time	Fig. 3.3.5.3(b)(i) & (ii)
	Design load and design strength of material to be adopted shall be 1.2 for concrete & 1.0 for steel	Table 3.3.5.1

Member dimensions & reinforcement requirements	
(a) Reinforcements of HS and NS walls	
(i) Landed development	Table 3.4.2.1 (a)
Minimum reinf. H10-100 c/c at both faces, both directions, max internal clear HS height 3900mm	
Shear link H8-600 c/c both directions	
Chear min to dee die best anodiene	
(ii) Non- Landed development	Table 3.4.2.1 (b)
Minimum reinf. H10-100 c/c at both faces, both directions, max internal clear HS/NS height (Ht): 2400 ≤Ht ≤ 3000mm	Table 0.4.2.1 (b)
Minimum reinf. H13-100 c/c at both faces, both directions, max internal clear HS/NS height (Ht): 3000 <ht 3900mm<="" td="" ≤=""><td></td></ht>	
Shear link H8-600 both directions	
(b) Reinforcements in HS and NS slab	Cl. 3.4.2.2
Minimum reinf. H10-100 c/c & shear link H8-600 c/c at both faces, both directions	01. 3.4.2.2
Millimum Term. n To-100 GC & Shear link no-600 GC at both faces, both directions	
Ceiling slab outside HS wall with HS door	Cl. 3.4.2.3
Min. 125mm thk with H10-100 c/c in both direction	01. 0.4.2.0
Min. 123mm tirk with A 10-100 C/C in both direction	
HS slab intergrated with footing/ pilecap	Cl. 3.4.2.4
	CI. 3.4.2.4
Max reinf. Spacing is 200 c/c and shear link provision not required for combine thickness >500mm.	
(a) Pointenance de la constate de la Marca III de 110 cm III	01.0.4.0.5
(c) Reinforcements in precast shielding wall for HS wall	Cl. 3.4.2.5
Shielding wall with air gap only permitted with HS wall without HS door	
Can be build of brick/ block wall /precast wall	Fig. 2.4.5 (a) - (g)
Precast concrete wall reinf. to be minimum H10-200 c/c mesh, shear links with H8-600 c/c with 75mm bend at two ends	
Air gap shall not be filled up	
Detailing of HS tower	
(a) Lap and anchorage length	
Full lap and anchorage length of reinf. to be provided according to concrete grade and bond condition	Table 3.5.2(a) & (b)
ruil lap and anchorage length of reini. to be provided according to concrete grade and bond condition	Table 5.5.2(a) & (b)
(b) Concrete cover to main reinforcement	Cl. 3.5.3
Minimum 25 mm; maximum 40mm	01. 0.0.0
William 20 min, maximum 40 min	
(c) Details of HS wall reinf. near HS door	Fig. 3.5.4(f)
Reinf, at each sides of blast door frame and its stiffener shall be 2H13	g. σ.σ(.)
Tream, at sacrification in place of place and marked simplified shall be 2.110	
(d)Ventilation sleeve trimmer bars	Fig. 3.5.5(h)
Additional trimmer bars around ventilation sleeve (to be placed within ventilation sleeve stiffenners), size of bar reinf. to follow wall reinf.	i ig. cross(r.y
(
(e) Shear links	Fig. 3.5.4 (i)
Hooks and bend must be tie to outermost reinf. of HS where the hook must always placed face internal face of HS	3 (.,
The same and the s	
(f) Precast hollow cores HS	
Size of precast hollow core to be in multiple of 100mm.	Table 3.5.6(c)
Min reinf. of reinf. bars in hollow cores depending on HS clear hieght.	13.3.5
Min 4H20 in 200mm cores, for max internal clear HS/NS height (Ht): 2400 <ht 3000mm<="" <="" td=""><td></td></ht>	
Min 4H20 in 200mm cores for max internal clear HS/NS height (Ht): 2400 ≤Ht ≤ 3000mm Min 4H25 in 200mm cores for max internal clear HS/NS height (Ht): 3000 <ht 3900mm<="" td="" ≤=""><td></td></ht>	

(g) Precast HS hollow core provision with HS wall with door	Fig. 3.5.6.1 (b)
All precast HS walls to be provided with hollow cores except for short HS wall with HS door of 1.2m wide HS (internal clear). HS door to be placed centrally Next permitted short HS wall with HS door is 1.35m (intermal clear), with 200mm core cage	
Minimum slab thickness and reinforced concrete topping to comply to requirement	Table 3.5.6(d)

Storey Shelter & Staircase Storey Shelter Structural Requirement Checklist:

- (1) The essential technical requirements that have impacts on the structural safety, performance and protection levels of the storey shelters are listed in the Table.
- (2) The list shall be used as a guide to check that the plan submitted fulfils essential technical requirements.
- (3) For other technical requirements not in the list, they shall be referred to Technical Requirements for Storey Shelters 2021.

Disclaimer: This checklist serves as a guide to qualified person in relation to the preparation of (i) Building Plans which will incorporate the architectural aspects of Civil Defence ("CD") Shelter Plans and (ii) Structural Plans which will incorporate the structural aspects of the CD Shelter Plans and are for informational purposes only. This checklist does not purport to be exhaustive or applicable to all situations and does not constitute professional advice. The Building and Construction Authority disclaims any liability (including any liability arising from negligence) arising in respect of any matter and the consequences of any act done or not done by any person in reliance on anything in or omitted from this checklist.

1	Material	Clause or Table or Fig. in TRSS 2021
	Concrete Minimum concrete grade: C32/40 Normal concrete density (2400kg/m³) consists of 20mm nominal maximum size coarse aggregate, apply to all SS/NS structures (include hollowcore & joints, RC elements within setback distance)	Cl. 3.2.1
	Steel reinforcement Welded steel fabric mesh for steel bar Yield stress min. 500 N/mm²	Cl. 3.2.2
2	Analysis	
	(a) Shielded NS walls (2 opposite NS wall [not applicable for S/C SS tower] or 4 NS walls) No additional design check if supporting NS wall/ NS columns are shielded by RC slab	Cl. 3.3.3 Fig. 3.3.3 and 3.3.4(c)
	(b) Unshielded NS walls (2 opposite NS wall [not applicable for S/C SS tower] or 4 NS walls) Min each NS wall 300 mm thk; Additional design check against the most severe effects of the removal of a portion of the NS wall equivalent to 1.5m opening (@ most critical position)	Cl. 3.3.4 (a)-(b) Fig. 3.3.4(a), (b) & (d)
	Design load and design strength of material to be adopted shall be 1.2 for concrete & 1.0 for steel	Table 3.3.4
3	Member dimensions & reinforcement requirements	
	(a)Reinforcements of SS and NS walls Minimum reinf. H13-100 at both faces, both directions, max internal clear SS/NS height (Ht): 2400 ≤Ht ≤ 3400mm Minimum reinf. H16-100 at both faces, both directions, max internal clear SS/NS height (Ht): 3400 <ht 3900mm="" both="" directions<="" h8-600="" link="" shear="" td="" ≤=""><td>Table 3.4.2.1</td></ht>	Table 3.4.2.1
	Common wall reinforcement : H10-100 c/c in both faces, shear link H8-600 c/c	
	Shielding wall in front of SS door Reinf. to be minimum H10-200 c/c mesh both faces, shear links with H8-600 c/c with L bend at two ends at both directions	

(b) Reinforcements in SS and NS slab	Cl. 3.4.2.2 (a) - (f)
Intermediate SS/NS slab and slabs/ waists of staircase SS/NS: Minimum reinf. H10-100 c/c & shear link H8-600 at both faces, both directions	
Ceiling slab of top-most SS: (300 mm minimum) Minimum reinf. H13-100 c/c & shear link H8-600 at both faces, both directions	
Floor slab of bottom-most SS or NS and floor slab of NS located above a SS: (300 mm minimum) Minimum reinf. H13-100 c/c & shear link H8-600 at both faces, both directions	
Ceiling slab outside SS tower immediately above SS door Min. 150mm thk with H10-100 in both direction Anchored into SS slab with tension anchorage length	
Floor slab outside SS tower to be structurally connected to the SS tower	
SS slab intergrated with footing/ pilecap Max reinf. Spacing is 200 c/c and shear link provision not required for combine thickness >500mm.	
4 Detailing of SS tower	
(a) Lap and anchorage length Full lap and anchorage length of reinf. to be provided according to concrete grade and bond condition	Table 3.5.2
(b) Concrete cover to main reinforcement Minimum 25 mm; maximum 40mm	Cl. 3.5.3
(c) Cast in Situ elements for SS and S/C SS All dimension and detailed requirements to follow Fig 35.4(a)-(m)	Cl. 3.5.4 Fig. 3.5.4(a)-(m)
(d) Details of SS wall reinf. near SS door Reinf. at each sides of blast door frame and its stiffener shall be 2H13	Fig. 3.5.4(d)(i), (ii)
(e) Ventilation sleeve trimmer bars Additional. trimmer bars around ventilation sleeve (to be placed within stiffeners), size of bar reinf. to follow wall reinf.	Fig. 3.5.4(f) Fig. 3.5.5 (a)(i)-(ii)
(f) Shear links Hooks and bend must be tie to outermost reinf. of SS where the hook must always placed face internal face of SS	Fig. 3.5.4 (h)
(g) Rescure hatches/ vertical / horizontal blast hatches Reinf. at each sides of blast hatch frame and its stiffener shall be 2H13	Fig. 3.5.4 (i), (j), (k)
h) Cat ladder Shock design calculations required. Comply to design for 12g, where g is the gravitational force To adopt CD approved anchor bolts with shock resistance	Cl. 2.11.2 Fig. 2.11.2

(i) Precast SS door frames Type 1 to 3	Fig. 3.5.5 (a)-(n)
Precast door frame fixed at 300mm, CIS connection fixed at 250mm.	
No ventilation sleeve to be placed at the CIS connection zone	
(j) Precast hollow core S/C SS	Cl. 3.5.6
	Table A & B of Fig. 3.5.6.1 (a)
Size of precast hollow core to be in multiple of 100mm	
Min reinf. of reinf. bars in hollow cores depending on SS clear height.	
SS/NS wall reinforcement provision as per internal clear height provision	Table 3.4.2.1
Min. 6 nos of rebars within the overlapping 'U' bars from precast staircase flight and cast-insitu landing slabs	
(k) Handrail mounting onto SS outer wall	
Shock design calculations to comply to design to withstand shock loads of 12.5g, where g is the gravitaional acceleration	
Shock design calculations should also take account of the R class value indicated in the manual.	
Shock design calculations should also take account of the R class value indicated in the manual. To adopt CD approved anchor bolts with shock resistance	
· ·	