

Requirements on piling plan submission

Under BC (Amendment) Act 2007 & Regulations 2008

Er Dr Yet Nai Song



Types of piling submissions



Bored cast-in place piles

Grouted micropiles



We shape a safe, high quality, sustainable and friendly built environment.

Barrettes

Types of piling submissions



Driven piles

Jacked-in piles



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1.1



Objective

 To help Qualified Persons who prepare the piling plans to comply with requirements of BC (Amendment) Act 2007 & Regulations 2008





Overview

- 1. Submission checklist
- 2. BC Regulations 10(2)
- 3. SS CP4 : 2003
- 4. BCA / IES / ACES Advisory Note on Site investigation & Load test
- 5. Piling Annexes B, C, D ■





(1) Submission checklist





Submission checklist

- I. <u>Piling plans</u> endorsed by QP and AC
- II. <u>Design reports</u> by QP and AC
- III. <u>Declaration forms</u> by QP and AC ►
- IV. Site investigation report, endorsed & certified by a PE
- V. <u>A plan</u> showing the layout and location of the site, boundary line and neighbouring buildings
- VI. Impact assessment report, where applicable (eg. for foundations located within influence zone of sensitive structures, like MRT or CST tunnels)
- VII. <u>Clearance letters</u>, where applicable, from the relevant authority



Certifications by QP and AC on plans

STANDARD CERTIFICATION ON STRUCTURAL PLANS AND DESIGN CALCULATIONS BY THE QUALIFIED PERSON FOR STRUCTURAL WORKS

- 1) In accordance with Regulation 9 of the Building Control Regulations I, ________ the Qualified Person for structural works appointed under section B(1)(a) or 11(1)(d)(i) of the Building Control Act, hereby submit the detailed structural plans and design calculations prepared by me and certify that they have been prepared in accordance with the provisions of the Building Control Regulations, the Building Control Act and any other written law pertaining to buildings and construction for the time being in force.
- I further certify that these detailed structural plans and design calculations are in reference to Project reference No:
- Total number of structural plans submitted: 2 Total number of pages of design calculations in this book: 390

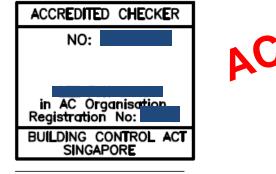






STANDARD CERTIFICATION ON STRUCTURAL PLANS AND DESIGN CALCULATIONS BY THE ACCREDITED CHECKER

- I, ______(ACCREDITED CHECKER) hereby certify that I have carried out an evaluation, analysis and review of the detailed structural plans and design calculations presented herein and am satisfied that there are no inadequacies in the key structural elements.
- Total number of structural plans checked : 2
 Total number of pages of design
 calculations checked in this book : 390
- I hereby declare that I have no professional or financial interest in the building works shown in the plans as defined in Section 18 of the Building Control Act (Cap 29)



(ACCREDITED CHECKER'S STAMP & SIGNATURE) Date: 04/08/2008

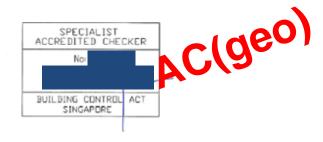
Certifications by QP(geo) and AC(geo) on plans

STANDARD CERTIFICATION BY THE SPECIALIST ACCREDITED CHECKER

I. Being a registered specialist accredited checker, hereby certify that I have in accordance with the Building Control (Accredited Checkers and Accredited Checking Organisations) Regulations carried out an evaluation, analysis and review of the plans of the underground building works attached, and to the best of my knowledge and belief the plans do not show any inadequacy in the geotechnical aspects relating to the underground building works if carried out in accordance with those plans.

2 I append my Evaluation Report (comprising 286, pages) as well as the analyses and calculations I have performed in carrying out the evaluation, analyses and review of the geotechnical reports and design calculations are in reference to Project Ref. No.: A second second

3 I hereby declare that I have no professional or financial interest in the underground building works shown in the plans as defined in Section 18 of the Building Control Act (Cap 29).



SPECIALIST ACCREDITED CHECKER'S SIGNATURE AND STAMP

Date: 2nd December 2008



STANDARD CERTIFICATION BY THE QUALIFIED PERSON FOR GEOTECHNICAL ASPECTS OF UNDERGROUND BUILDING WORKS

In accordance with Regulation 10A of The Building Control Regulations I, <u>Er.</u>, the Qualified Person for the geotechnical aspects of the underground building works appointed under section 8(1)(d)(i) or 11(1)(d)(iii) of the Building Control Act, hereby submit the underground building works plans and the design calculations

prepared by me and certify that, to the best of my knowledge and belief, they have been prepared in accordance with the provisions of the Building Control Regulations, the Building Control Act and any other written faw pertaining to buildings and construction for the time being in force.

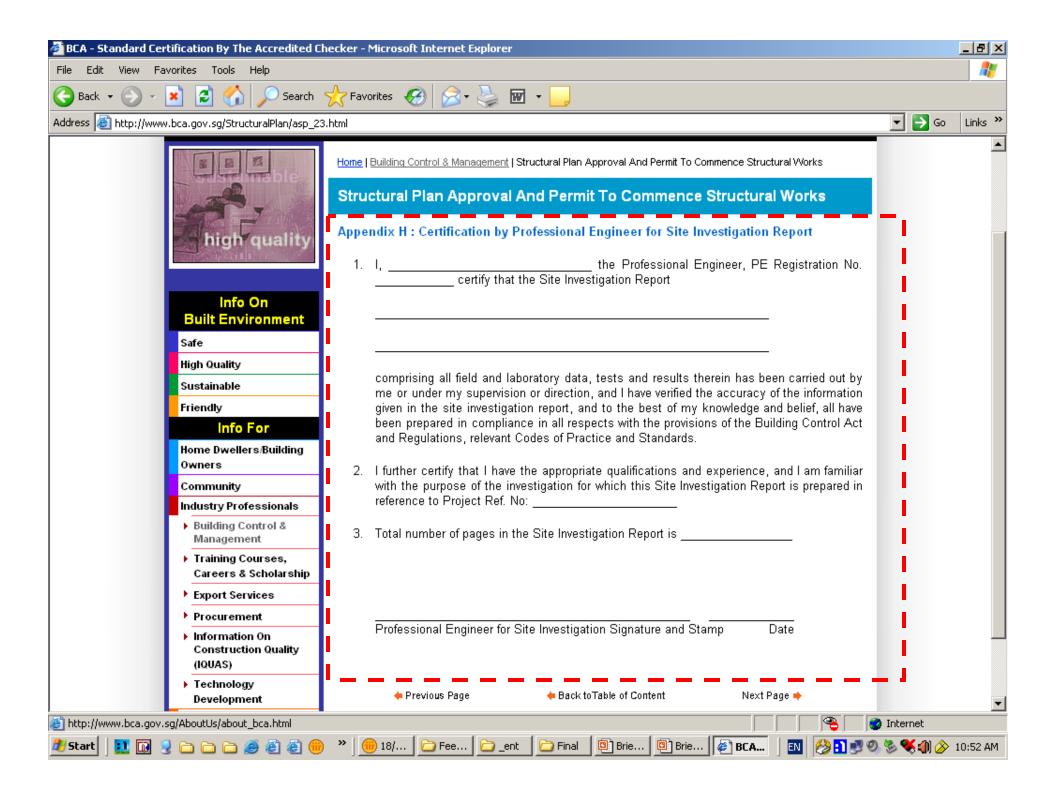
2 I further certify that these underground building works plans and design calculations are in reference to Project Ref. No;

3 Total no. of underground building works plans submitted; <u>7</u> Total no. of pages of design calculations in this book; <u>201</u>



QUALIFIED PERSON'S SIGNATURE AND STAMP Date :

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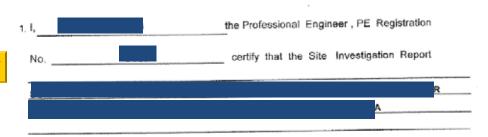
Certification by PE for SI Report

Certification by Professional Engineer For Site Investigation Report

Certification by PE for SI report







Comprising all field and laboratory data, tests and results therein has been carried out by me or under my supervision or direction, and I have verified the accuracy of the information given in the site investigation report, and to the best of my knowledge and belief, all have been prepared in compliance in all respects with the provisions of the Building Control Act and Regulations, relevant Codes of Practice and Standards.

2. I further certify that I have the appropriate qualifications and experience and I am familiar with the purpose of the investigation for which this Site Investigation Report is prepared in reference to Project Ref. No:

3. Total number of pages in the Site Investigation Report is

353

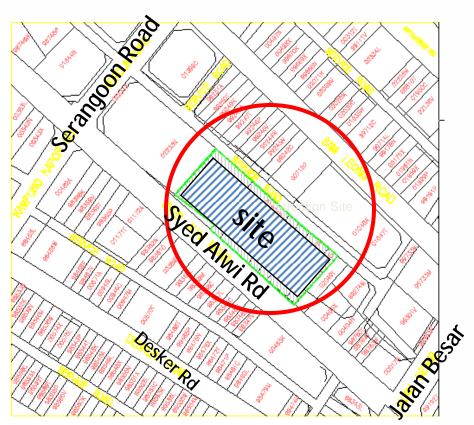


Professional Engineer for Site Investigation Signature and Stamp

2-9/11/2008



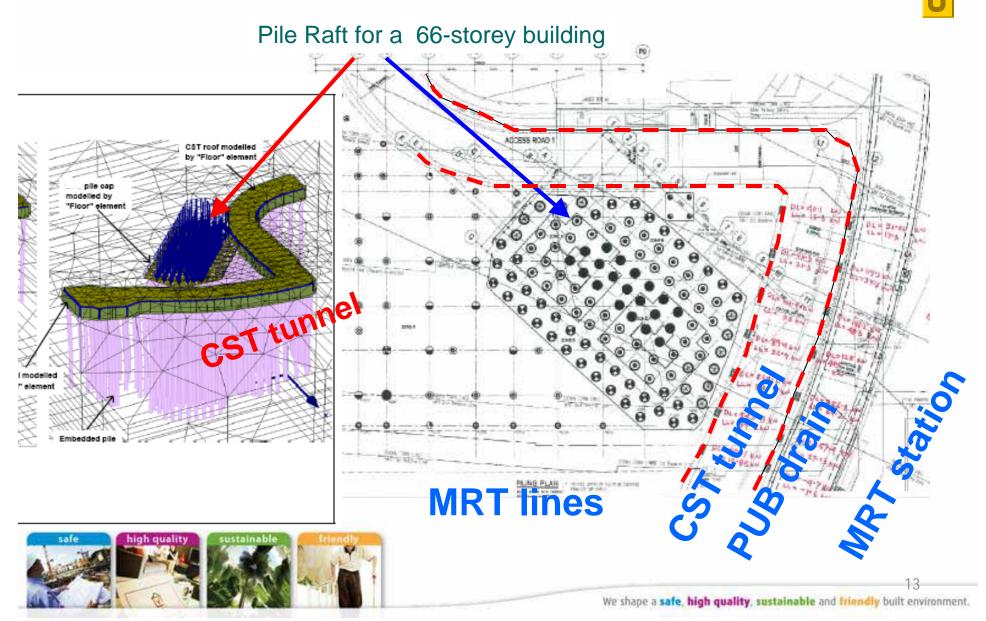
Plan to show Site Location



TION PLAN



An example of project close to sensitive structures



Building and Construction Authority



(2) Building Control Regulation 10(2)







- (2) The pile layout plans shall show
 - (a) the types of piles and the specification of materials to be used;
 - (b) the location of piles;
 - (c) the estimated pile penetration depth;
 - (d) the allowable pile bearing capacity before and after deduction of negative skin friction (if applicable) and details of pile joints; and
 - (e) the sectional details of piles and number and type of pile load tests.



Regulation 10(2)

- - (a) the types of piles and the specification of materials to be used;
 - (b) the location of piles and site investigation boreholes ;
 - (c) the estimated pile penetration depth for each design zone ;
 - (d) the minimum embedded pile length into competent stratum, where applicable;
 - (e) the unit skin friction and unit end bearing resistance for pile designs;
 - (f) the allowable pile bearing capacity before and after deduction of negative skin friction (if applicable) and details of pile joints;
 - (g) the allowable total and differential foundation settlement;
 - (h) the allowable vibration limit during pile installation; and
 - (i) the sectional details of piles and number and type of pile load tests and the location of ultimate pile load tests.

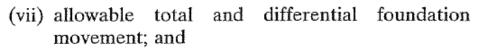


Building and Construction Authority

Regulations 2008 – Reg 10A(5) (Plans of foundation ≥ 30storeys)

(5) The foundation plans for the design and construction of foundation for buildings of 30 or more storeys shall contain, where applicable —

- (a) the layout, sections and details of all foundation works showing
 - (i) types of piles or foundation and specification of material to be used;
 - (ii) location of piles or foundation and site investigation boreholes;
 - (iii) pile or foundation founding depth or pile minimum embedment into competent stratum for each pile or foundation;
 - (iv) unit shaft friction, pile base resistance or foundation bearing pressure;
 - (v) allowable foundation capacity before and after accounting for negative skin friction where applicable, allowable tension, and lateral load;
 - (vi) details of pile reinforcements, pile joints, connection with pilecap, pile shops;



- (viii) allowable vibration limit; and
- (b) the number, type of pile or foundation tests, structural integrity tests and location of preliminary test pile or ultimate load tests and site investigation for the tests.





Building Control Regulations 2008 Regulation 10(2)

- (2) The pile layout plans shall show
 - (a) the types of piles and the specification of materials to be used;
 - (b) the location of piles and site investigation boreholes ;
 - (c) the estimated pile penetration depth for each design zone; \triangleright
 - (d) the minimum embedded pile length into competent stratum, where applicable;
 - (e) the unit skin friction and unit end bearing resistance for pile designs;
 - (f) the allowable pile bearing capacity before and after deduction of negative skin friction (if applicable) and details of pile joints;
 - (g) the allowable total and differential foundation settlement; ▶
 - (h) the allowable vibration limit during pile installation; and
 - (i) the sectional details of piles and number and type of pile load tests and the location of ultimate pile load tests.





(a) Pile type & material specification, pile joints, sectional details

- I. Do the plans show the <u>pile types</u> and <u>material specification</u>?
- II. Do the plans show <u>pile size</u> and <u>pile details</u>?
- III. Do the plans show the pile capacity ?

(i) <u>allowable foundation capacity</u>, before and after accounting for negative skin friction,

(ii) allowable tension load, lateral load, where applicable.

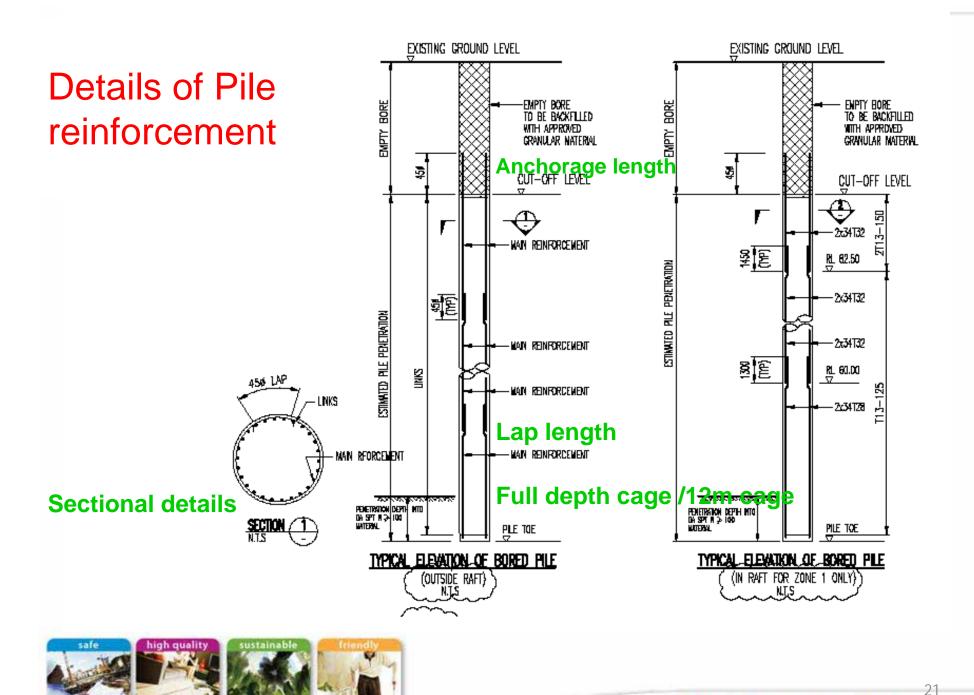


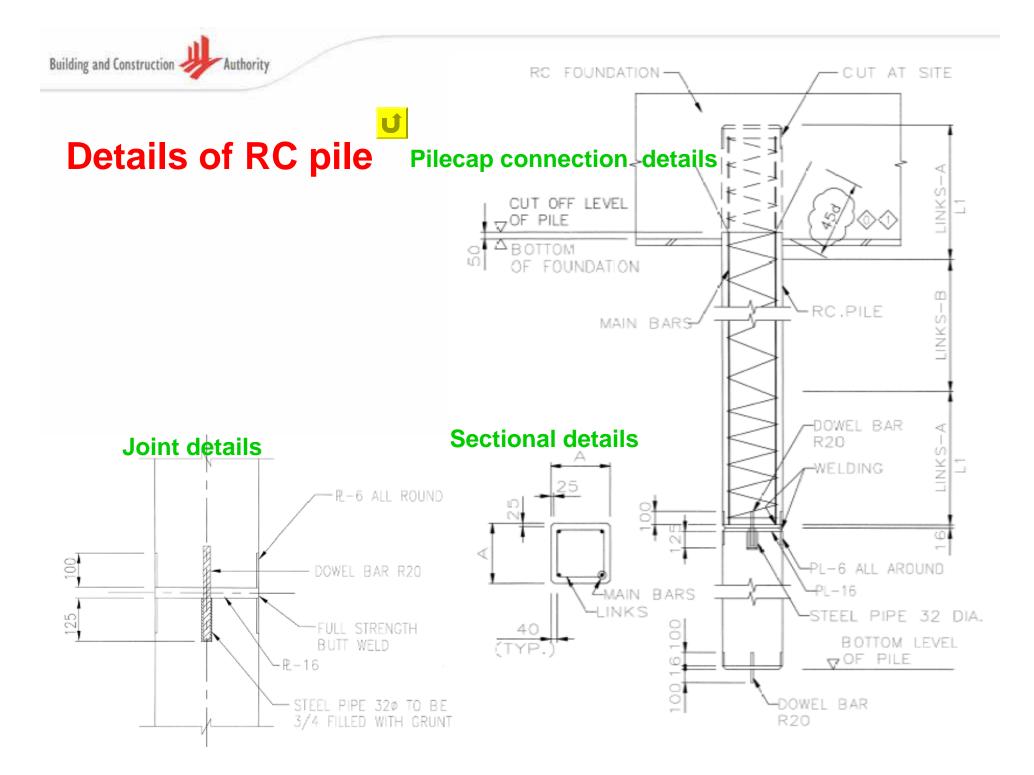
Items to be shown on piling plan

Pile capacity & Pile reinforcement

					Norkir	ng loa	C	k		Pile	e reinfo	orcem	ent
PIU	NG 31	, MERAFI	2 (OUTSIE	JE KAFIJ									
ALE TYPE	LEC ENO	NÊ ÂT	NO. OF FILES	OEPTH OF ENPTY BORE	WORIDING LOAD [HI]					NNNUN Eirstich dêmi	NAIN Reinforcement	LINKS	
		(nn)		(m)	CONFRESSION	TENSKIN		(RON COL(m))	INTO	SPT N>100(m			
A	Θ	≉1000	4	11.5	5850	-		51.0		[6.5]	17T25	T10-175	
A(T)	$ \oplus$	¢1000	4	11.5	5850	1600		51.0		6.5	20125	T10-175	
B	$\mathbf{\Phi}$	¢1100	-	11.5	7100	-		52.0		7.5	20125	T10-175	
C		ø1200	4	11.5	8450	-		53.0		8.5	24725	T10-175	
C(T)	\\$	ø1200	8	11.5	8450	2500		54.0		8.5	24125	T10-175	
D	\oplus	ø1.300	$\left\{ \begin{array}{c} 4 \end{array} \right\}$	11.5	9950	-		54.0		9.5	22128	T1D-175	
D(T)	\oplus	ø1300	L.	11.5	9950	2500		5 5.0		9.5 }	22128	T10-175	
E	Ð	ø1400	6	11.5	11500	-		5 5.0		10.5	26128	T10-175	
F	•	≠ 1500	6	11.5	13250	-		56.5		11.5	29728	T10—175	
G	Œ	¢1 600	5	11.5	15000	-		57.5		12.5	33T28	T10-175	
н	\odot	¢1 <i>8</i> 00	6	11.5	19000	-		60.0		{ <u>14.5</u> }	32132	T10-175	









(b) Site investigation borehole

- I. Do the plans show location of site investigation boreholes?
- II. Is there any borehole within the footprint of each block ?

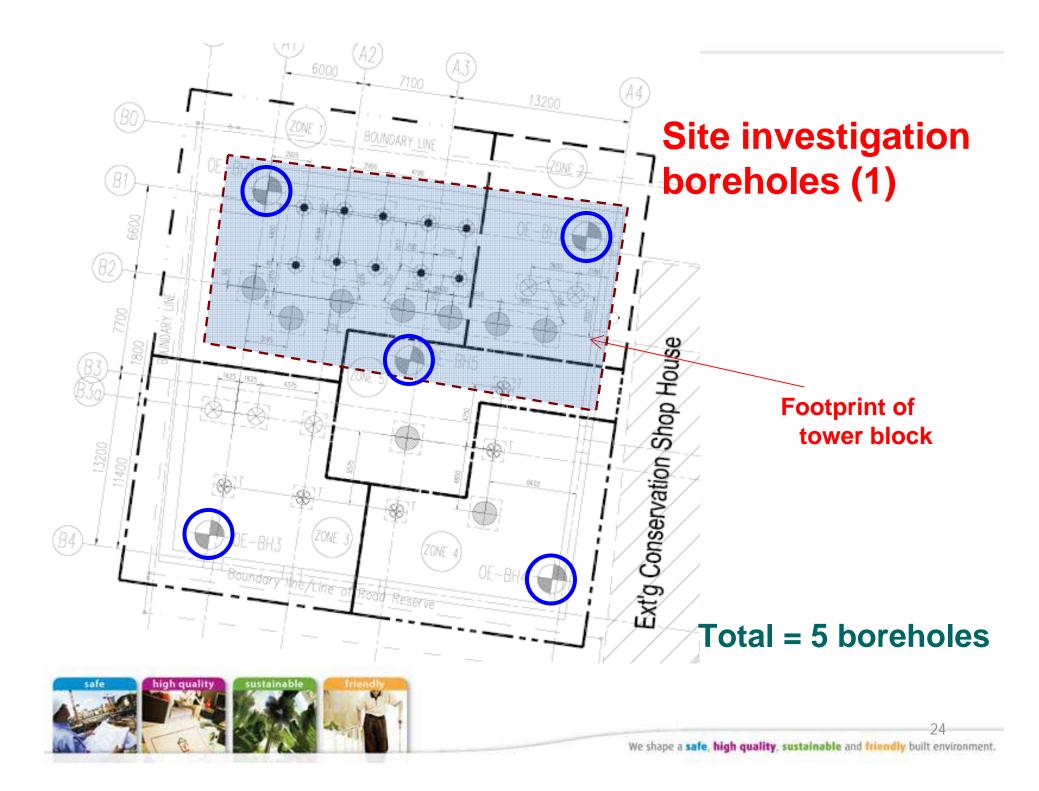
For buildings of 10-storeys or more

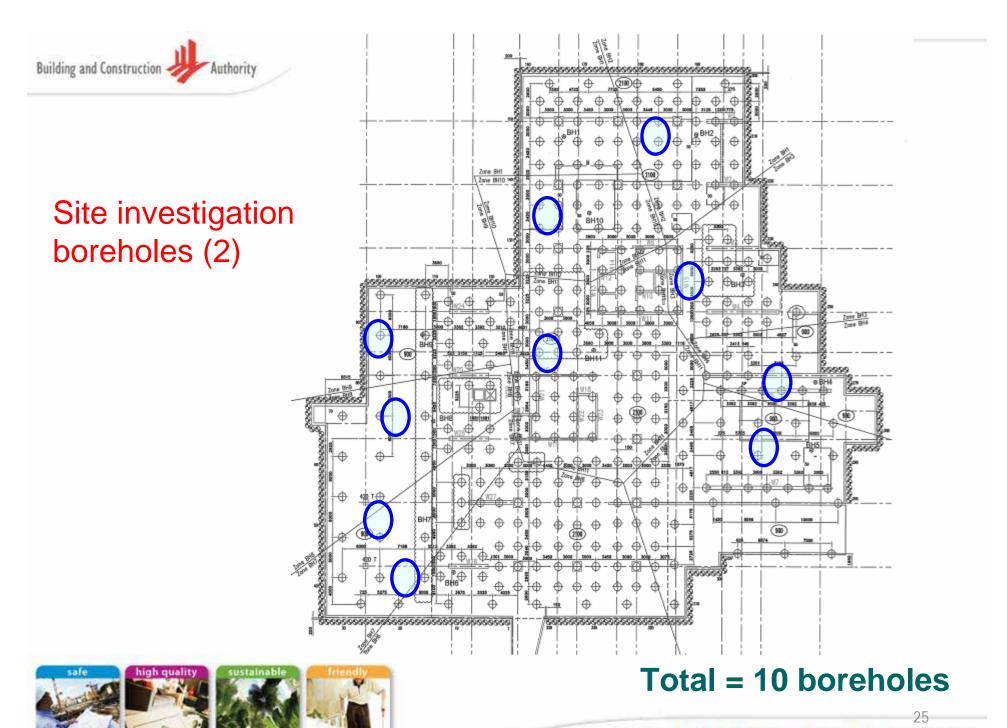


- III. Do <u>number of boreholes</u> exceed minimum ?
- IV. Is <u>depth of any borehole</u> beyond proposed pile penetration depth? (see BCA/IES/ACES advisory note 2003)



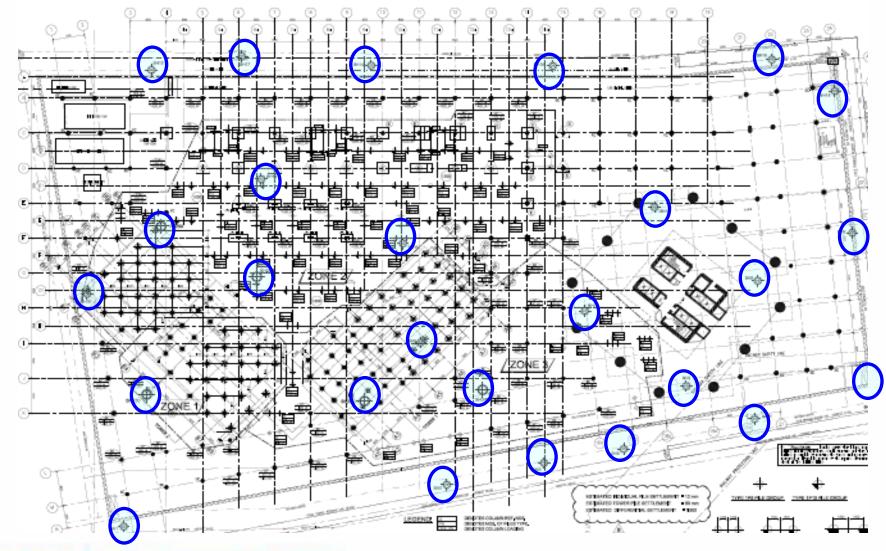
Items to be shown on piling plan







Site investigation boreholes (3)







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- I. Do the plans show <u>pile design zones</u> (based on site investigation information) ?
- II. Do the plans show the following information ?

(i) <u>pile founding criteria</u> (eg. pile founding depth / minimum embedment into competent stratum / pile set criteria / magnitude of jack in load)

(ii) a description of soil / rock for founding layer.



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Pile design zones based on boreholes

BOR	ed pile	E (GRADI	E 35) ((FOR COM	PRESSIO	n Piles)										
	BORED	PLE	REINFTS.	REINFTS.	SPIRAL	TOTAL NO.	ESTI	NATED PEN			ram cut—al	ff-level)				
PILE TYPE	PILE	CAPACITY	PROMOZED		UNKS	OF PLES	Zone 1 BH1		ZDNE 2 BH2 & BH3		ZONE 3 BH4		ZONE 4 BH5		ZONE 5 BH6	
	SEE															
							PL.	SL (o)	PL	SL (a)	PL.	SL (a)	PL.	SL (a)	PL	SL (0)
А & В 🕀	¢60D	212 T	9 T 16	12 m	T10-175		21.2	-	19.5	3.1	18.5	5.6	22.3	-	20.D	4.6
° 💮	ø70D	2881 T	11 T 16	12 m	T10-175		23.2	-	21.0	4.6	20. D	7.1	22.5	0.2	21.0	5.6
□ ●	\$800	377 T	13 T 16	12 т	T10-175		25.7	-	22.0	5.6	21.0	8.1	24.0	1.7	22.5	7.1
E D	¢ 90D	477 T	13 T 20	12 m	T1D-200		26.2	-	23.0	6.6	22.0	9.1	25.0	2.7	23.5	8.1
F	¢1000	589 T	15 T 20	12 m	T1D-20D		26.5	0.3	24.5	8.1	23.5	10.6	26.0	3.7	24.5	9.1
•	¢1100	712 T	17 T 20	12 m	T1D-200		27.7	1.5	25.5	9.1	24.5	11.6	27.5	5.2	26.D	10.6
*	¢1200	848 T	19 T 2D	12 m	T10-200		28.7	2.5	26.5	10.1	25.5	12.6	28.5	6.2	27.D	11.6
' 🏵	¢1300	995 T	22 T 20	12 m	T10-200		30.2	4.0	27.5	11.1	27.0	14.1	29.5	7.2	28.D	12.6
PL= PILE LENG SL= SOCKET LI	TH FROM C	ut-off-level	SOIL)	ĥ	TUTAL							-				-

Example 1



Pile founding criteria (in hard soil)

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Example 2 Pile founding criteria (in hard soil & rock)

\sim	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~						
BH No.	EST. LENGTH OF PILE (FROM COL)m	PILE FOUNDING CRITERION						
1	19.1	14.5m OF HARD SILT (SPT>100) (RESIDUAL SOIL)						
2	24.4	6.275m OF HARD SILTY SAND (SPT>100) (COMPLETELY WEATEHRED SANDSTONE)						
3	20.7	1.8m OF HARD CLAYEY SILT (SPT>100) (RESIDUAL SOIL)						
4	32.0	3.5m OF HARD CLAYEY SILT (SPT>100) (RESIDUAL SOIL)						
5	17.1	2.3m OF HARD SILT (SPT>100) AND 0.2m OF MODERATELY WEATHERED SANDSTONE (SOCKETING)						
6	15.8	2.35m OF VERY STIFF SILT WITH WEATHERED ROCK, SILTSTONE (SPT>100) AND 4.5m OF MODERATELY WEATHERED SILTSTONE (SOCKETING)						
7	9.8	4m OF HARD HIGHLY WEATHERED SILT (SPT>100) AND 2m OF MODERATELY WEATHERED SILTSTONE (SOCKETING)						
8	20.1	7m OF HARD SILT WITH CLAY (SPT>100)						
9	15.8	4.85m OF HARD SILT WITH CLAY AND PIECES OF SILTSTONE (SPT>100) AND 3.5m OF MODERATELY TO HIGHLY WEATHERED, HIGHLY FRACTURED SILTSTONE (SOCKETING)						
10	18.8	9.775m OF HARD CLAYEY SILT/SILTY CLAY (SPT>100)						
11	20.1	8.275m OF HARD SILTY CLAY (SPT>100)						





Example 3 Pile founding criteria (pile set)

THAT MEASURES THE PEAK PARTICLE VIBRATION VELOCITY. SETTLEMENT MONITORING BY THE USAGE OF SETTLEMENT MARKERS. CRACK MONITORING BY THE USAGE OF TELL-TALE GLASS.

20. ALL PRESTRESSED PRECAST SPUN PILES SHALL BE DRIVEN TO A FINAL SET OF 15M FOR THE LAST10 BLOWS. THIS SET IS CALCULATED FROM THE HILEY'S FORMULA BASED ON THE PARAMETERS AS SPECIFICED BELOW. ef WH [W + n²Wp] Ou = $[S+0.5(C_1+C_2+C_3)][W+Wp]$ WHERE Ou = ULTIMATE PILE CAPACITYef = HAMMER EFFICIENCY = 100% W = HAMMER WEIGHT = 10 Ton H = HAMMER DROP HEIGHT = 750mm n = COEFFICIENT OF RESTITUTION = 0.4 Ć Wp = WEIGHT OF PILES = SET = 2.0 mm / BLOW $C_1 + C_2 + C_3 = \text{TEMPORARY COMPRESSION} = 14 \text{mm}$ SAFETY FACTOR = 3.0WHERE THE METHOD OF PILE DRIVING & EQUIPMENT USED DIFFER FROM THOSE ASSUMED. CONTRACTOR SHALL PROPOSE SUBJECT TO SO'S APPROVAL THE SETS OF THE PILES SO THAT THE PILES CAN ACHIEVE THE DESIGNED WORKING LOAD CAPACITIES.

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21. THE TYPE OF PILING FRAME USED ON SITE SHALL BE IN ACCORDANCE WITH ANY RELEVANT AUTHORITIES' REQUIREMENTS. DIESEL PILING HAMMERS ARE NOT PERMITTED IN URBAN AREAS.



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(e) Geotechnical parameters

- I. Do the plans contain <u>unit shaft friction</u> and <u>unit</u> <u>pile base resistance</u> used for pile design ?
- II. Are soil strata / rock type given for the above ?

		* SOIL DESIGN PARAMETERS								
NOTES FOR BORED PILES; ALL PARAMETERS TO BE CONFIRMED BY INSTRUMENTAL SUTINATE PILE LOAD TEST		ZONE 1	(BH4)	ZONE 2 (BH3)						
11) design parameter : UNIT SKIN FRICTION = 2.0 N (WHEN N<100 SOIL) UNIT SKIN FRICTION = 250 KN/m ² (WHEN N≥100 SOIL) UNIT END BEARING = 7000 KN/m ² (WHEN N≥100 SOIL)	SOIL DESCRIPTION	SKIN FRICTION fs (kN/m^2)	END BEARING fb (kN/m²)	SKIN FRICTION fs (kN/m²)	END BEARING fb (kN/m²)					
12) SAFEIT FACTOR :	, di s ^a nne									
1.5 (SKIN FRICTION) + 3.0 (END BEARING) 2.5 (SKIN FRICTION) + 2.5 (END BEARING) 1.5 (FOR SKIN FRICTION ONLY)	FIRM TO STIIFF SILTY CLAY VERY STIFF SILTY CLAY	12 80		93						
13) FOUNDATION SETTLEMENT:	VERY STIFF SANDY CLAY	90		-						
TOTAL BUILDING SETTLEMENT LIMIT = 25mm WAX. DIFFERENTIAL SETTLEMENT = 1/500	VERY STIFF CLAYEY SILT HARD CLAYEY SILT STONE	- 200		127	9000					
14) MINIMUM SPACING BETWEEN SPUN PILE TO BE 2.5 # UNLESS OTHERWISE S (15) THE VIBRATION LIMIT FOR PILING IS <u>3 mm/sec</u>)	VERY HARD CLAYEY SILT STONE	200	9000	-	9000					
	<u>Note:</u> * verified by instrumented ul.'	t load test								

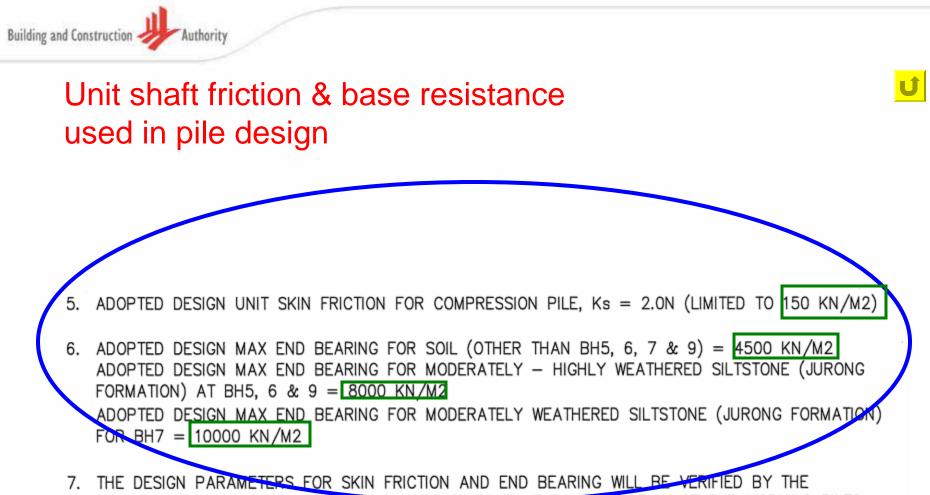


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Authority

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Items to be shown on piling plan



ULTIMATE LOAD TEST CARRIED OUT ON SITE PRIOR TO THE INSTALLATION OF THE WORKING PILES.





(f) Negative skin friction

I. Is there a <u>design evaluation</u> of the possibility of negative skin friction ?

(i.e. whether the soil is still undergoing consolidation at its final stress state ?)

II. Has negative skin friction been <u>considered</u> in the design ?



Items to be shown on piling plan

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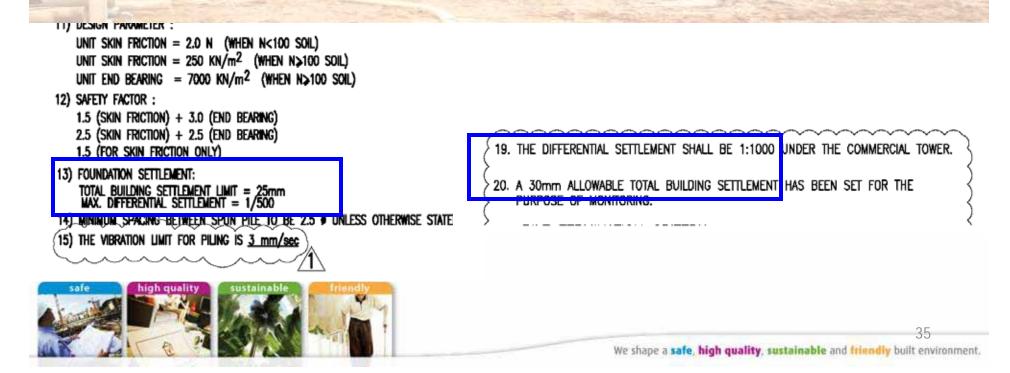
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(g) Allowable settlements

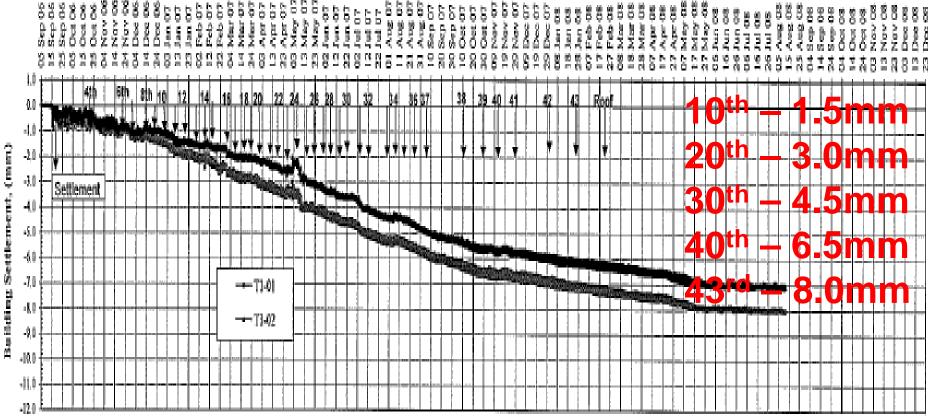
Are allowable total and differential foundation settlements shown on plan ?

II. Are these allowable foundation settlements consistent with the design calculation ?





Sample variation of building settlement with time during superstructure erection



Settlement increases with building height.



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(h) Vibration limits

Are allowable vibration limits specified on the plan ?

NOTES FOR BORED PILES:

ALL PARAMETERS TO BE CONFIRMED BY INSTRUMENTAL ULTIMATE PILE LOAD TEST.

11) DESIGN PARAMETER :

UNIT SKIN FRICTION = 2.0 N (WHEN N<100 SOIL)

- UNIT SKIN FRICTION = 250 KN/m² (WHEN N>100 SOIL)
- UNIT END BEARING = 7000 KN/m^2 (when N>100 soil)

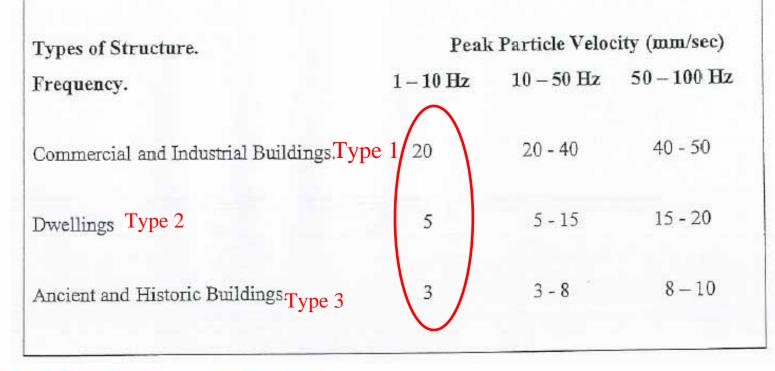
12) SAFETY FACTOR :

- 1.5 (SKIN FRICTION) + 3.0 (END BEARING)
- 2.5 (SKIN FRICTION) + 2.5 (END BEARING)
- 1.5 (FOR SKIN FRICTION ONLY)
- 13) FOUNDATION SETTLEMENT:
 - TOTAL BUILDING SETTLEMENT LIMIT = 25mm MAX. DIFFERENTIAL SETTLEMENT = 1/500
- 14) MINIMUM SPACING BETWEEN SPLIN PILE TO BE 2,5 # UNLESS OTHERWISE STATED.
- (15) THE VIBRATION LIMIT FOR PILING IS <u>3 mm/sec</u>

Building and Construction

DIN 4150 : Part 3 "Structural vibration in buildings" Guideline on Limit of vibration

DIN 4150







(i) Verification tests

I. Do the plans contain the following information ?

(i) <u>number of tests</u>,

(ii) types of pile tests.

- II. Are types and number of tests provided complied with BCA/IES/ACES advisory note 2003 ?
- III. Is location of ultimate pile load test shown on plan, and correlated with borelog ?
- IV. Will the preliminary piles for the ultimate pile load test be <u>instrumented</u>, to verify the geotechnical parameters ? (Applicable to piles where strain gauges could be installed along the pile, eg. bored piles, hollow spun pile)





- 10. GEOTECHNICAL CAPACITY OF THE PILE SHALL BE TAKEN AS THE LESSER OF Qs/2 OR (Qs+Qb)/2.5, WHERE Qs IS THE ULTIMATE SHAFT RESISTANCE AND QU IS THE ULTIMATE BASE RESISTANCE. NEGATIVE SKIN FRICTION SHALL BE CONSIDERED IN THE PLING DESIGN.
- 11. WORKING LOAD TEST REQUIREMENTS:

NEGATIVE SKIN FRICTION SHALL BI		N.
WORKING LOAD TEST REQUIREMEN		NLT + 9PDA
TEST DESCRIPTION	NUMBER OF PILES TO BE PLETEP 2 NO OF #1500 3 NOS OF #1800	min: +9PDA
ULTIMATE LOAD TEST	2 NO OF #1500	mum - A
WORKING LOAD TEST	3 NOS OF #1800	
	1 NO OF #1200	
	1 NO OF #1000	
	5 NOS OF #1800	
HIGH STRAIN DYNAMIC TEST (PDA TEST)	1 NOS OF \$1500	
	1 NOS OF #1400	
	1 NOS OF #1200	
	1 NOS OF #1000	
PILE INTEGRITY TEST (PIT TEST)	10 NOS (ANY SIZE)	

AUDURT AFTAILED LICTUAR ATATELIENTA





Preliminary pile to be instrumented

	1 NOS OF #1000
PILE INTEGRITY TEST (PIT TEST)	10 NOS (ANY SIZE)

- THE CONTRACTOR SHALL SUBMIT DETAILED METHOD STATEMENTS FOR BORED PILE INSTALLATION AND THE CARRYING OUT OF PILE TESTING.
- 13. THE BASE OF THE PILE SHALL BE THOROUGHLY CLEAN, SOUND AND FREE FROM LOOSE SAND DEPOSIT, LAITANCE, SILT PRIOR TO CONCRETING. THE CONTRACTOR SHALL CLEAN THE BASE USING AIR LIFT AND/OR OTHER APPROVED METHOD.

14. MINIMUM REINFORCEMENT LAD SHALL DE 45 TIMES THE DAD DIAMETER.

- 15. ULTIMATE PILE LOAD TEST SHALL BE CONDUCTED ON FULLY— INSTRUMENTED PILES WITH AT LEAST ONE LEVEL OF FOUR VIBRATING WIRE STRAIN GAUGES NEAR THE PILE TOE AND AT EACH CHANGE OF SOIL STRATUM, BUT NOT MORE THAN 5 METRES BETWEEN ANY TWO LEVELS. THE VERTICAL SPACING OF THE STRAIN GAUGES SHALL NOT BE MORE THAN 3m IN THE SPT N > .100 SOIL LAYER, THE CONTRACTOR SHALL ALSO ALLOW FOR THE INSTALLATION OF THREE ROD EXTENSIONETERS IN EACH OF THE ULTIMATE TEST PILES.
- 16. THE CONTRACTOR SHALL CARRY OUT PRE-CONDITION SURVEYS ON ADJACENT BUILDING IN FULL COMPLIANCE WITH THE SPECIFICATIONS. THE COST OF CARRYING OUT PRE-CONDITION SURVEYS IS DEEMED TO BE INCLUDED IN THE TENDER.
- 17. THE BASE OF EACH PILE SHALL BE FULLY GROUTED.







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Arecap

Items to be shown on piling plan

- a) Pile type & material specification, pile joints, sectional details
- **b)** Site investigation boreholes
- c) Estimated pile length for each design zone
- d) Minimum embedded depth into competent stratum

- e) Skin friction & End bearing for pile design
- f) Pile capacity before & after negative skin friction
- g) Allowable settlements
- h) Allowable vibration limits
- i) Pile load test & location





(3) SS CP4 : 2003

(Code of Practice for Foundations)



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SPRING singapore

SINGAPORE STANDARD CP 4: 2003 (ICS 91.040; 93.020)

code of practice for Foundations

Published by SPRING Singapore 2 Bukk Morah Central Singapore 158335 SPRING Singapore Webstle: www.spring.gov.ag Standards Webstle: www.standards.org.ag



Code of Practice CP4 : 2003





Design for pile foundations [CP4 Clause 7.3.1]

- Every pile design has to satisfy <u>3 conditions</u>:
 - 1. Adequate safety factor against failure
 - 2. Settlement of foundation under working load should not affect the serviceability of structure
 - 3. Safety of nearby buildings should not be put at risk





Pile Group Effect (CP4 Clause 7.3.4)

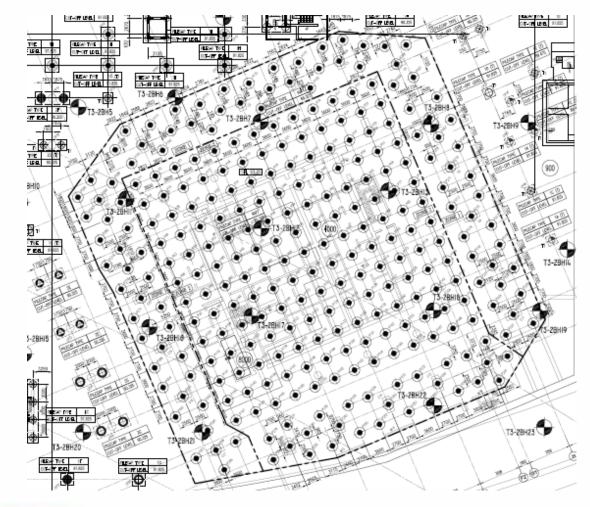
- I. Group settlement is greater than settlement of individual pile
- II. Differential settlement between two pile groups < 1:500



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An example where pile group effect is dominant with overlapping of stress bulbs of individual piles

300 nos 1.8m piles within 60mx60m footprint





Effect of lateral forces during basement excavation (CP4 Clause 7.4.5.3.2)

MAAAA

maaaa

MAAAA

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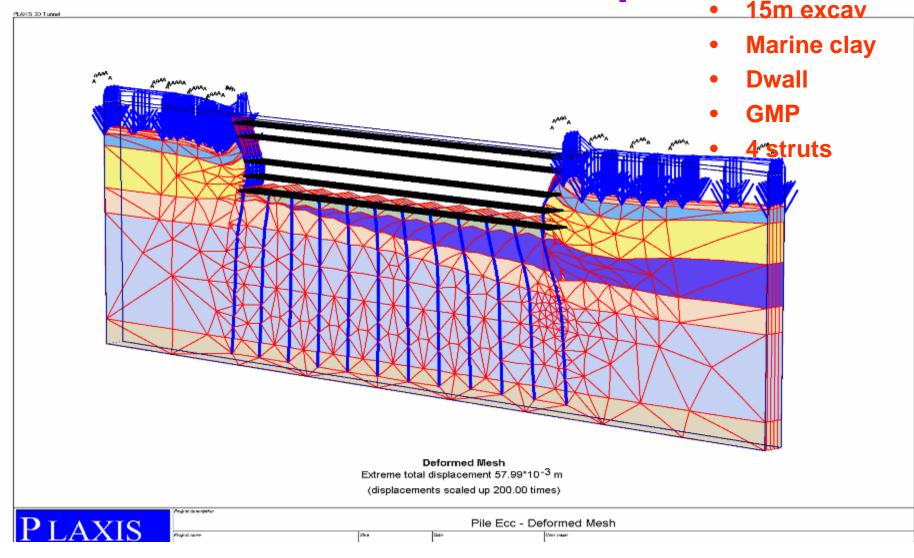
If bored piles are subject to lateral forces, reinforcement should be adequately provided for bending moments. • to full pile length or

to where bending moment is negligible.

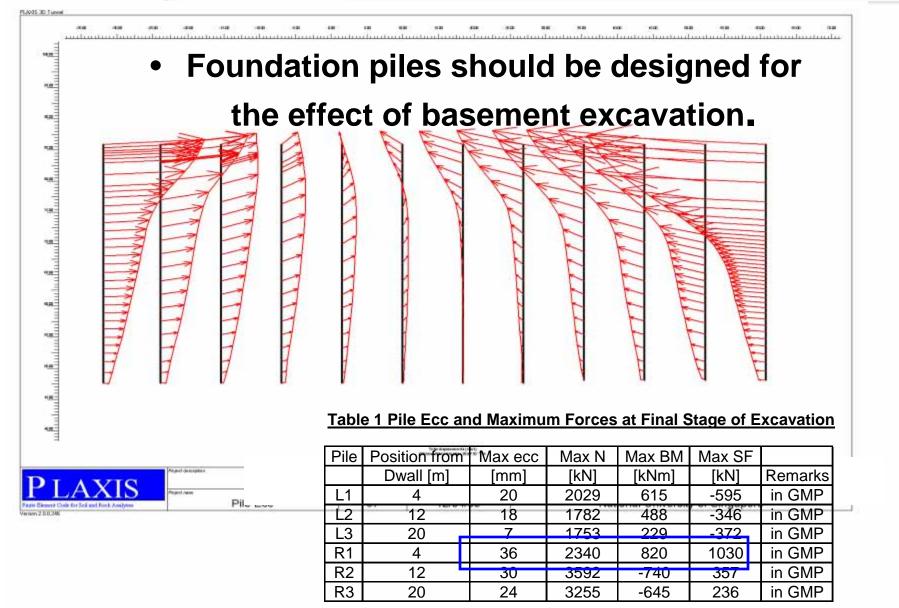
KIS 3D Tunne



An example of effect of basement excavation on foundation piles



it.



Foundation piles move by 36mm, and experience bending moment of 820 kNm !





(4) BCA / IES / ACES Advisory note

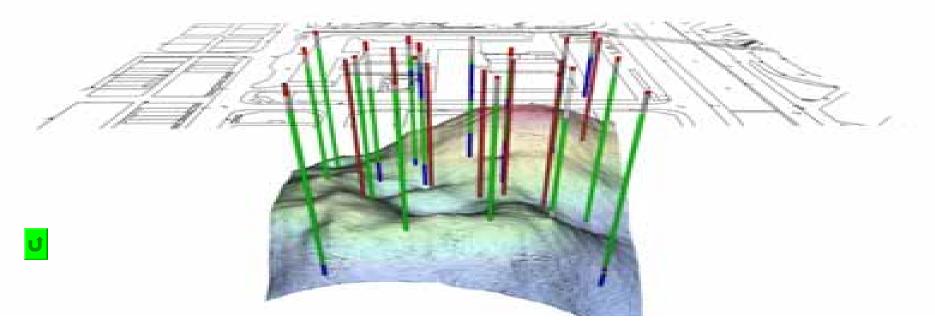
(for buildings of 10-storeys or more)



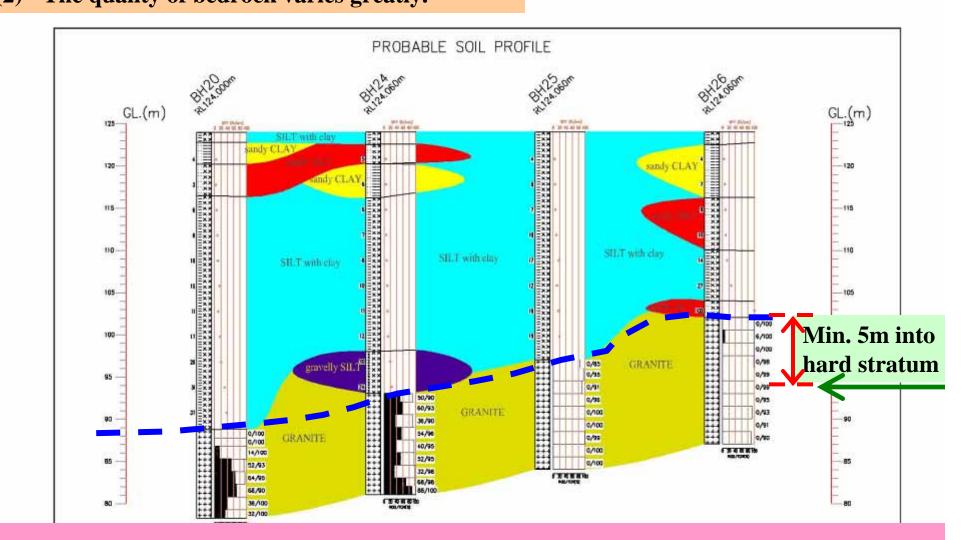


Requirements for Site Investigation to establish the soil strata and ground variation

	Minimum requirement	
(a) Number of boreholes	 1 borehole per 300 m² 1 borehole at 10 to 30m spacing Minimum 3 boreholes 	
(b) Depth of boreholes	 - 5 metre into hard stratum with N>100 - 3 times pile diameters beyond pile founding level. 	



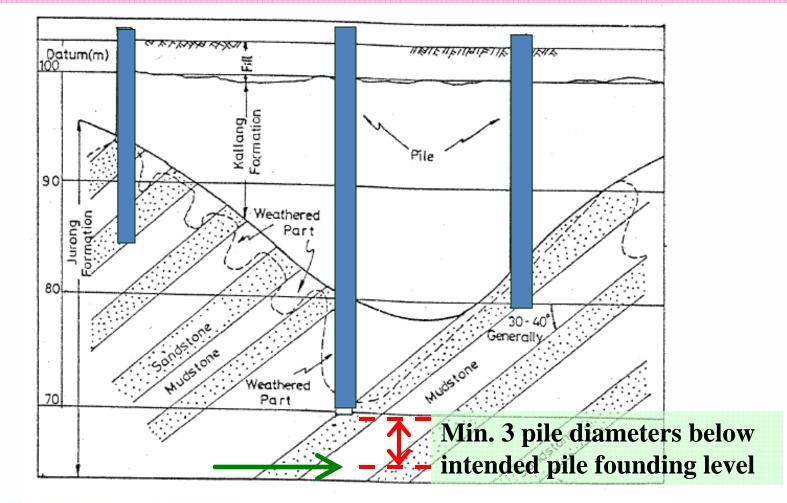
Rockhead varies from RL 102m to RL88m The quality of bedrock varies greatly.



Example of soil profile for Bukit Timah Formatio



Example of soil profile for Jurong Formation







Type of Load Test	Pile Test Schedule
(a) Ultimate load test on preliminary pile (preferably instrumented)	1 number or 0.5% of the total piles whichever is greater.
(b) Working load test	2 numbers or 1% of working piles installed or 1 for every 50 metres length of proposed building, whichever is greater.
 (c) Non-destructive integrity test. (Note: This is for the purpose of quality control, and high-strain type should be used for bored piles) 	2 numbers or 2% of working piles installed, whichever is greater.



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Load measurement for pile load tests (CP4 Clause 7.5.5.2.1)

 Load should be measured by a calibrated load gauge & a calibrated pressure gauge







(5) Piling Annexes

(for buildings of 10-storeys or more)





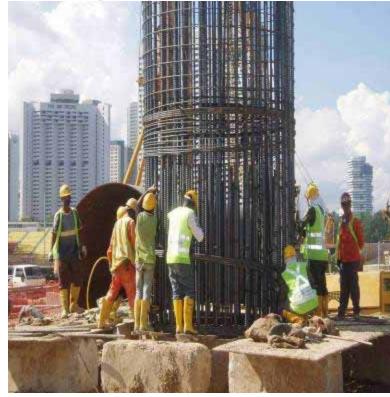
Piling Annex B Certificate of supervision of pile load test

	Commissioner of Building Control Building and Construction Authority 5 Maxwell Road #05-00 Tower Block, MND Complex Singapore 069110	INSTRUCTIONS 1 * Delete accordingly 2 Please use BLACK INK to complete the form				
	Particulars of Structural Works to which this Certificate relates					
	ST					
	*Lot / Plot : *TS / MK :					
	Address/Road :					
	Project Title :	· · · · · · · · · · · · · · · · · · ·				
	Pile load test ref. no. : Meth	od of testing : Date of test :				
	Part A: To be completed by the Qualified Person	for Structural Works / # Qualified Person for Geotechnical Aspects				
	not faulty,	l cell, pressure and dial gauges, and they are properly calibrated by SPRING				
	 (c) I have implemented the measure to prevent m (d) All recordings of load and settlement reading 					
	 (a) An recording of rotating schemen reducings are winessed of the site subject visor, (e) I have examined the test records for the *static pile load test and I am satisfied that the load test result is valid and acc (f) The test pile is loaded to a maximum of <u>important</u> times working load and it has *passed/failed the load-test chierion specifies (g) I am satisfied with the pile load test results, the load test together with the site records confirmed the vester assumptions parameters made in the pile design 					
	(f) The test pile is loaded to a maximum of (g) I am satisfied with the pile load test results,	times working load and it has *passed/failed the load-test diterion specified.				
	 (f) The test pile is loaded to a maximum of	times working load and it has *passed/failed the load-test editerion specified, the load test together with the site records confirmed the testion assumptions of				
	 (f) The test pile is loaded to a maximum of	times working load and it has *passed/failed the load-test editerion specified, the load test together with the site records confirmed the test on assumptions of the load test together with the site records confirmed the test of the test of the test of the test of the test				
	 (f) The test pile is loaded to a maximum of	times working load and it has *passed/failed the load-test editerion specified, the load test together with the site records confirmed the test on assumptions of the load test together with the site records confirmed the test of the test of the test of the test of the test				
	 (f) The test pile is loaded to a maximum of	time's working load and it has *passed/failed the load-test chiterion specified, the load test together with the site records confirmed the test sign assumptions of al Works # Signature & Name of On lines Person for Geotechnical Aspects Data accordance to CLA 2008, ng of test approach, e.g. load cell, pressure and dial gauges before and durin				
	 (f) The test pile is loaded to a maximum of	time's working load and it has *passed/failed the load-test chiterion specified, the load test together with the site records confirmed the test on assumptions of al Works # Signature & Name of On lines Petron for Geotechnical Aspects Data accordance to CLR 20, 8, ng of test of presence e.g. load cell, pressure and dial gauges before and durin, s arc vines ped by the qualified person or the site supervisor.				
	 (f) The test pile is loaded to a maximum of	time's working load and it has *passed/failed the load-test chiterion specified, the load test together with the site records confirmed the test sign assumptions of al Works # Signature & Name of On lines Person for Geotechnical Aspects Data accordance to CLA 2008, ng of test approach, e.g. load cell, pressure and dial gauges before and durin				
	 (f) The test pile is loaded to a maximum of	time's working load and it has *passed/failed the load-test chiterion specified, the load test together with the site records confirmed the test on assumptions of al Works # Signature & Name of On lines Petron for Geotechnical Aspects Data accordance to CLR 20, 8, ng of test of presence e.g. load cell, pressure and dial gauges before and durin, s arc vines ped by the qualified person or the site supervisor.				
	 (f) The test pile is loaded to a maximum of	The second se				
	 (f) The test pile is loaded to a maximum of	The second se				
	 (f) The test pile is loaded to a maximum of	The second se				

THE BUILDING CONTROL ACT (Cap 29)



Piling Annex C Interim certificate of supervision of piling works





THE BUILDING CONTROL ACT (Cap 29)			
INTERIM CERTIFICATE OF SUPERVISION ON PILING WORKS [Regulation 23(1) of The Building Control Regulations]			
Commissioner of Building Control Building and Construction Authority 5 Maxwell Road #05-00 Tower Block, MND Complex Singapore 069110	INSTRUCTIONS 1 * Delete accordingly 2 Please use BLACK INK to complete the form		
Particulars of Structural Works to which this Certificate relates			
Project ref. no.:	ST		
*Lot / Plot :	*TS / MK :		
Address/Road :			
Building Name (if any) :			
Project Title :			
Part A: To be completed by the Qualified Person for Structure	al Works / # Qualified Person for Geotechnical Aspects		
I, she Qualified Person appointed under * Section 8(1)(b) / Section 8(1)(d(ii) / Section 11(1)(d)(ii) / Section 11(1)(d)(iii) of the building Control Act for structural works certify that *(50%/100%) piling works have been completed on			
Signature & Name of Qualified Person for Structural Works	# Signature & Name of Qualified Person & Atechnical Aspects		
Part B: To be completed by the Builder	June -		
I confirm that I have sought the qualified person(s)'s approval for the founding depths of all the piles installed and they are constructed in accordance with the approved set of plan provided by the qualified person(s).			
Signature & Name of Builder Date :	Name & Address of Builder Firm		
Part C: To be completed by the Site Supervisor (s)			
I confirm that I have sought the qualified person(s)'s confirmation on the founding depths of all the piles installed.			
Signature & Name of Site Supervisor (s)	stisor		
Date : Site SU	pervisor		
# Only applicable for underground building works			

Annex C

PABE011002: Piling Advice Letter

Annex D

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Piling Annex D Certificate for monitoring building

settlement





THE BUILDING CONTROL ACT (Cap 29)

CERTIFICATE OF MONITORING BUILDING SETTLEMENT [Regulation 37 of The Building Control Regulations]

Commissioner of Building Control Building and Construction Authority 5 Maxwell Road #05-00 Tower Block, MND Complex Singapore 069110	INSTRUCTIONS 1 * Delete accordingly 2 Please use BLACK INK to complete the form		
Particulars of Structural Works and building to which this Cert	tificate relates		
Project ref. no.:	ST		
*Lot / Plot :	Lot / Plot : *TS / MK :		
Address / Road :			
Name / block number of building:			
Project Title :			
To be completed by the Qualified Person for Structural Works / # Qualified Person for Geotechnical Aspects			
I confirm that the above building has reached store	ys on(date) under my supervision.		
above building do not exceed the design limits in accordance	am satisfied that building settlement monitoring results so far for the ce with the approved set of structural plans/calculations, together with relevant provisions stipulated under the Building Control Act and its		
Signature & Name of Qualified Person for Structural Works	# Signature & Name of Qualified Person for Geotechnical Aspects OP (BOD) Date :		
# Only applicable for underground building works			



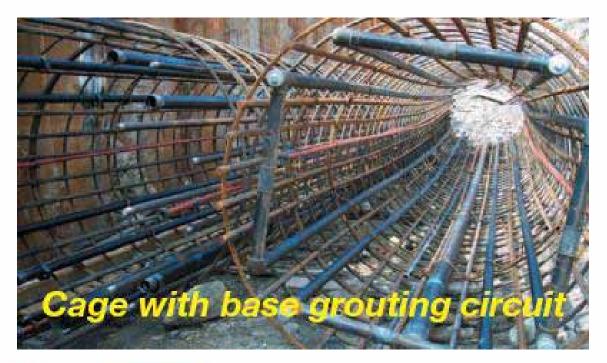
(6) Some good practices in pile design & construction





a) Reduce over-reliance on pile base resistance

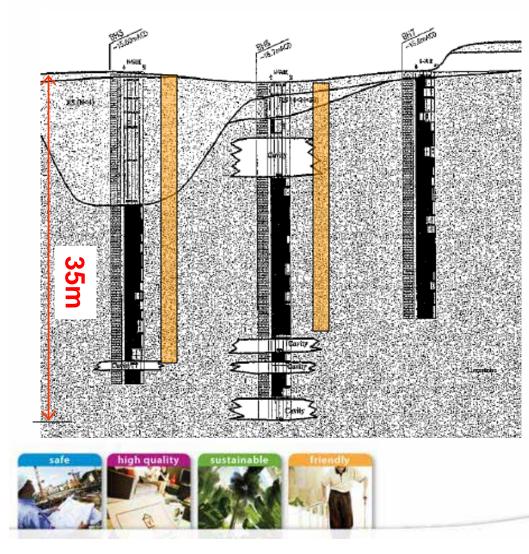
1) Applying a minimum safety factor to skin friction, for bored piles, where soft toe may occur





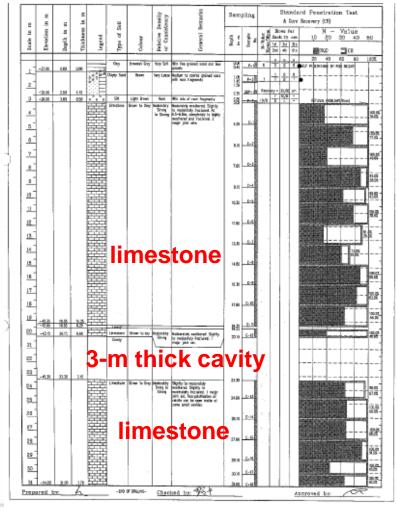
a) Reduce over-reliance on pile base resistance

2) In erractic soil condition, eg limestone area



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(b) Preliminary pile at borehole location





(c) Judicious location of strain gauges

- Taking into account :
 - change of soil layers
 - thickness
 - variability

• To aid interpretation eg. measurement of skin friction at various soil /rock layers





(d) Innovative Investigative preliminary pile



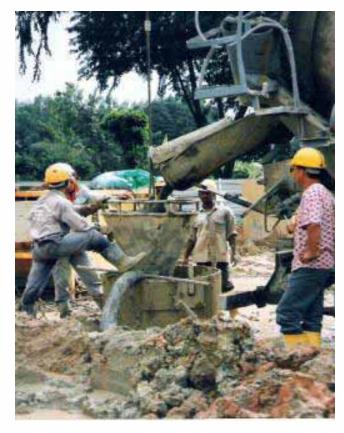
CP4 Clause 7.5.4.1

 Preliminary piles may be formed with voids at base or soft toes to determine ultimate shaft resistance





(e) Investigative preliminary piles installed using similar construction method as working production piles





CP4 Clause 7.5.4.1

Wet hole method





(f) Proper transfer of load to Kentledge frame

Use stiffener for effective transfer



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g) Minimum distance between test pile & Kentledge leg

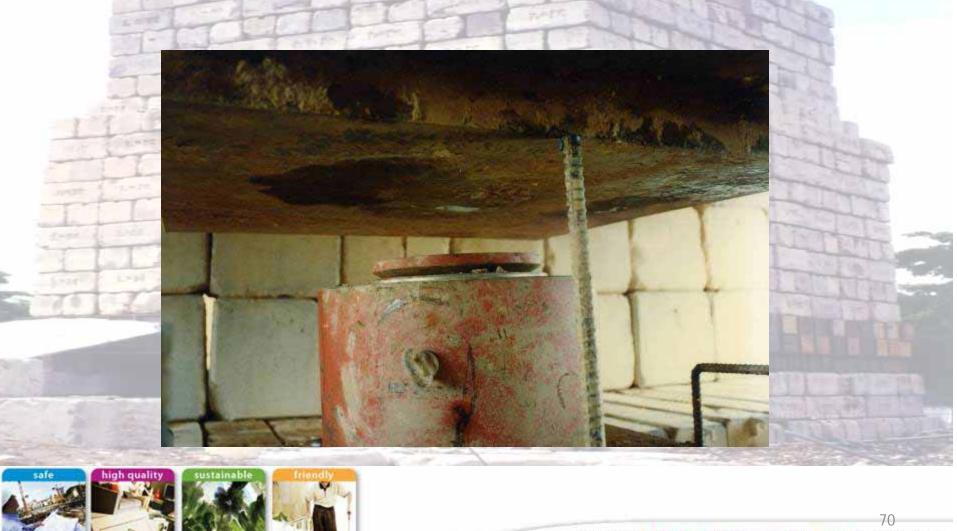
 Distance between Kentledge leg to surface of test pile ≥ 1.3 m (CP4 Clause 7.5.5.2.1)







(h) No preloading on piles before test



(i) Conduct working load test at location where it matters

Pile Size	Working Load	Nos	No. of	
(mm)	(kN)		Load Test	
ASM	-	135	0	
1000	5800	147	2	
1200	8400	99	1	
1500	13200	115	2	
1800	19000	159	3	
2000	23500	96	1	
2200	28500	88	1	
2500	36800	62	1	
2800	41600	22	1	
3000	53000	16	Evam	ples :
				pies.

939

Total :

(1) Piles within the tower footprint(2) Piles carrying larger load



(j) No preselection of pile for working load test



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(k) Keep rock/soil sample at site for verification



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1) Pile bore stability

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- 2) Pile base cleanliness
- 3) Tests on stabilizing fluids







(I) Impose quality control measures at site



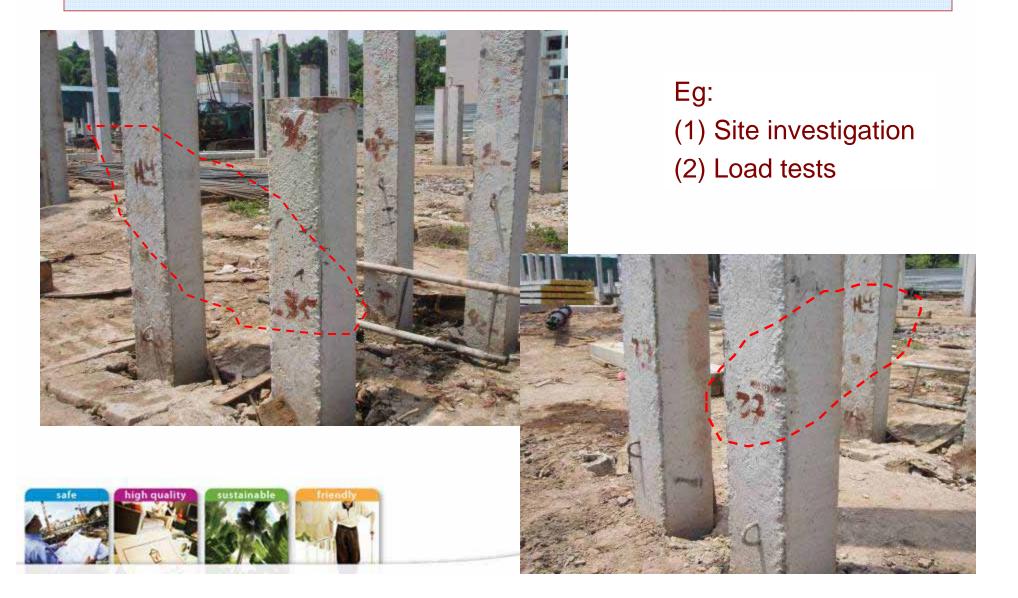


4) Sufficient cooling time for weld to gain strength





(m) Check & verify obvious change in pile depth



Recap of Some Good Practices

- a) Reduce over-reliance on pile base resistance
- b) Preliminary pile at borehole location
- c) Judicious location of strain gauges
- d) Innovative preliminary pile

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- e) Preliminary pile installed using same method as working piles
- f) Proper load transfer to Kentledge frame
- g) Minimum distance between test pile & Kentledge leg



- h) No preloading on piles before test
- i) Conduct working load test at location where it matters
- j) No preselection of pile for working load test
- k) Check & verify obvious change in pile depth
- Keep soil/rock samples at site for verification
- m) Impose quality control at site





- 1. Requirements of piling plan submission under recent BC amendment Act and Regulations
- 2. Requirements for foundations of buildings more than 10-storeys
- 3. Good practices in design, load tests & construction





Concluding remark

Standard Certification by QP on plans and calculations "I certify that they have been prepared in accordance with the provisions of the Building Control Regulations, the Building Control Act and any other written law pertaining to buildings and construction for the time being in force."

• QP who prepare plans and calculations for piling plans have to comply with the requirements of BC Act and regulations





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